

## The Consequences of Chloride-Induced Corrosion on Steel Bar and Reinforced Concrete Structures

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It is widely known that corrosion of steel reinforcement is one of the most important durability issues in reinforced concrete design [1]. The corrosion phenomenon can be initiated due to chloride ingress in concrete or due to depassivation of the protective thin oxide film of the steel reinforcement, through the action of carbon dioxide from the atmosphere. The corrosion activity degrades not only the appearance of the structure, but also its strength and safety, with the subsequent reduction in the cross sectional area, the strength and ductility properties of the reinforcement and with the decrease of bond with the surrounding concrete [2,3]. In coastal regions where high chloride concentrations are encountered, chloride-induced corrosion is the major source of environmental deterioration of reinforced concrete structures.

Chlorides that reach the concrete surface, enter the pore system either by diffusion (in stationary pore water), or by capillary suction of the surface water in which they are dissolved (or by combination of both transport mechanisms).

Chloride attack is distinct in that its primary action is the corrosion of steel reinforcement and only as a consequence of the corrosion process the surrounding concrete is damaged. Corrosion propagation is taking place at a rate that depends on the availability of both oxygen and water at the cathode. However, even at low rates of  $O_2$ , severe pitting corrosion has been noticed. It can be attributed to the fact that the anodic sites may be localized but the corresponding cathodic sites may be spread out over a wide area [1,4-7]. In this way, as the corrosion product is discouraged from precipitation, and due to the existence of highly active and localized anodic sites, a severe pitting corrosion may occur without an earlier warning through visible signs at the surrounding concrete. This can lead to rapid loss of the load bearing capacity of the reinforced concrete member.

The majority of the related studies on the effects of corrosion refer to steel bars directly exposed to the corrosive environment (bare bars), usually under salt-spray exposure [8,9]. Aspects, as the subsequent mass loss, the depth and the density of the pits formed on its surface are usually evaluated.

What is however neglected to a great extent, even by the appropriate standards, is the evaluation of how the accumulated corrosion alters the mechanical properties of the steel bar. In the relevant national standard as well as in the other appropriate European National Standards, the ability of a steel bar for mechanical performance is considered as being unchanged during the entire lifetime of a reinforced concrete structure. Even though it is early recognized that chloride induced corrosion, characterized by the continuous occurrence of pitted regions on the steel reinforcement, it leads to substantial reductions in the bar cross-section. Such type of damage can have significant effects on strength and ductility of reinforced concrete elements especially in dual-phase steel of high yield and ductility demand [2,3,8-15]. Existing design codes for steel reinforced structures do not take into consideration the accumulated material damage due to corrosion since they are unable to quantify it and need further review.

Given the above, apart from a few valid efforts investigations on the effects of corrosion on the mechanical performance of reinforcing, steel bars remains rare. To this extent, the influence of accumulated corrosion damage of steel bars embedded in concrete on the reduction of bond strength between bar and concrete has been investigated and assessed [2,3,8-15].

The frame degradation of a structure from reinforced concrete (i.e. bridges) that is located within an earthquake prone region and a coastal environment seems to be time dependant, since the damage accumulation due to corrosion as much as the damage accumulation due to low cycle fatigue are time dependant functions. Based on the damage accumulation in steel, the reconsideration of useful life appears to be necessary as the possible exceed of damage can seriously affect the integrity of structures.

In all events, the evaluation of the strength and design of corroded steel using the properties of original material may lead to over-design and erroneous results with potential catastrophic results.

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